



East Devon District Council Level 2 Strategic Flood Risk Assessment Detailed Site Summary Tables





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Fluvial plus climate change	<p>extremely unlikely this this depth of flooding would occur on site across the Flood Zone 2 extent. It is therefore likely that Flood Zone 2 is not a fair representation of the Flood Zones on site and development can proceed within Flood Zone 2, avoiding Flood Zone 3 to the east.</p> <p>As there is no detailed modelling available for this site, Flood Zone 3a has been used as a proxy for Flood Zone 3b. Detailed modelling should be undertaken as part of a detailed site-specific Flood Risk Assessment to define the extent of Flood Zone 3b and confirm the extent of Flood Zone 3a and Flood Zone 2.</p> <p>In the absence of detailed modelling, the Risk of Flooding from Surface Water dataset has been used to assess the depth, hazard and velocity flood risk to the site. Consideration should still be given to the Flood Zones and detailed modelling may be required within a site specific assessment.</p>																								
Surface Water	<p>Available data and mapping: Environment Agency's Risk of Flooding from Surface Water dataset for the 3.3%, 1% and 0.1% AEP events.</p> <p>Coly_06 - Surface Water 3.33% AEP - Depth Coly_06 - Surface Water 3.33% AEP - Hazard Coly_06 - Surface Water 3.33% AEP - Velocity Coly_06 - Surface Water 1% AEP - Depth Coly_06 - Surface Water 1% AEP - Hazard Coly_06 - Surface Water 1% AEP - Velocity Coly_06 - Surface Water 0.1% AEP - Depth Coly_06 - Surface Water 0.1% AEP - Hazard Coly_06 - Surface Water 0.1% AEP - Velocity</p> <p>Data analysis:</p> <p>3.3% AEP (1 in 30 year) event:</p> <table style="width: 100%; border: none;"> <tr> <td style="width: 50%;">Proportion - <1%</td> <td style="width: 50%;">Mean Depth - 0.21m</td> </tr> <tr> <td>Max Depth - 0.29m</td> <td>Mean Velocity - 0.1m/s</td> </tr> <tr> <td>Max Velocity - 0.18m/s</td> <td>Mean Hazard - 0.71</td> </tr> <tr> <td>Max Hazard - 0.96</td> <td></td> </tr> </table> <p>1% AEP (1 in 100 year) event:</p> <table style="width: 100%; border: none;"> <tr> <td style="width: 50%;">Proportion - 1%</td> <td style="width: 50%;">Mean Depth - 0.12m</td> </tr> <tr> <td>Max Depth - 0.36m</td> <td>Mean Velocity - 0.38m/s</td> </tr> <tr> <td>Max Velocity - 0.6m/s</td> <td>Mean Hazard - 0.63</td> </tr> <tr> <td>Max Hazard - 1.19</td> <td></td> </tr> </table> <p>0.1% AEP (1 in 1000 year) event:</p> <table style="width: 100%; border: none;"> <tr> <td style="width: 50%;">Proportion - 17%</td> <td style="width: 50%;">Mean Depth - 0.12m</td> </tr> <tr> <td>Max Depth - 0.46m</td> <td>Mean Velocity - 0.7m/s</td> </tr> <tr> <td>Max Velocity - 1.58m/s</td> <td>Mean Hazard - 0.65</td> </tr> <tr> <td>Max Hazard - 1.29</td> <td></td> </tr> </table>	Proportion - <1%	Mean Depth - 0.21m	Max Depth - 0.29m	Mean Velocity - 0.1m/s	Max Velocity - 0.18m/s	Mean Hazard - 0.71	Max Hazard - 0.96		Proportion - 1%	Mean Depth - 0.12m	Max Depth - 0.36m	Mean Velocity - 0.38m/s	Max Velocity - 0.6m/s	Mean Hazard - 0.63	Max Hazard - 1.19		Proportion - 17%	Mean Depth - 0.12m	Max Depth - 0.46m	Mean Velocity - 0.7m/s	Max Velocity - 1.58m/s	Mean Hazard - 0.65	Max Hazard - 1.29	
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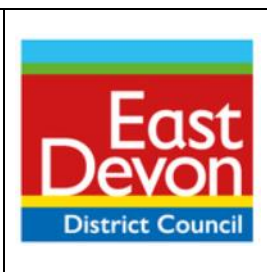




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	<p>Flood characteristics: During the 3.3% AEP event less than 1% of the site is shown to be at risk of flooding in the northeastern corner with a maximum depth of 0.29m. Within the 1% AEP event, a flow path forms from the centre of the site out to the east, increasing in extent out to the west during the 0.1% AEP event. It is therefore evident that as the flooding extent increases, the average flood depth decreases. The mean depth, velocity and hazard within the 0.1% AEP event are shown to be 0.12m, 0.7m/s and 0.65 (a 'caution') respectively.</p>																								
Surface water plus climate change	<p>Available data and mapping: Environment Agency's Risk of Flooding from Surface Water dataset for the 3.3%, 1% and 0.1% AEP events with 65% Climate Change scenarios.</p> <p>Coly_06 - Surface Water 3.33% AEP plus 65% Climate Change - Depth Coly_06 - Surface Water 3.33% AEP plus 65% Climate Change - Hazard Coly_06 - Surface Water 3.33% AEP plus 65% Climate Change - Velocity Coly_06 - Surface Water 1% AEP plus 65% Climate Change - Depth Coly_06 - Surface Water 1% AEP plus 65% Climate Change - Hazard Coly_06 - Surface Water 1% AEP plus 65% Climate Change - Velocity Coly_06 - Surface Water 0.1% AEP plus 65% Climate Change - Depth Coly_06 - Surface Water 0.1% AEP plus 65% Climate Change - Hazard Coly_06 - Surface Water 0.1% AEP plus 65% Climate Change - Velocity</p> <p>Management Catchment: Coly_06 is located within the East Devon Management Catchment. The Environment Agency guidance recommends that the Upper End allowance is considered for both the 3.3% and 1% AEPs for the 2070's epoch, unless the allowance for the 2050's epoch is higher, in which case this should be used. This is appropriate for development with a lifetime beyond 2100. The recommended uplift on peak rainfall intensity for the 3.3% AEP is 40% and for the 1% AEP is 45%. As Risk of Flooding from Surface Water data with a 65% uplift was already available this has been used as the best available data for the 3.3%, 1% and 0.1% AEPs.</p> <p>Data analysis:</p> <p>3.3% AEP (1 in 30 year) plus 65% climate change event:</p> <table style="width: 100%; border: none;"> <tr> <td>Proportion - 5%</td> <td>Mean Depth - 0.11m</td> </tr> <tr> <td>Max Depth - 0.43m</td> <td>Mean Velocity - 0.55m/s</td> </tr> <tr> <td>Max Velocity - 0.96m/s</td> <td>Mean Hazard - 0.61</td> </tr> <tr> <td>Max Hazard - 1.24</td> <td></td> </tr> </table> <p>1% AEP (1 in 100 year) plus 65% climate change event:</p> <table style="width: 100%; border: none;"> <tr> <td>Proportion - 16%</td> <td>Mean Depth - 0.12m</td> </tr> <tr> <td>Max Depth - 0.49m</td> <td>Mean Velocity - 0.65m/s</td> </tr> <tr> <td>Max Velocity - 1.54m/s</td> <td>Mean Hazard - 0.64</td> </tr> <tr> <td>Max Hazard - 1.33</td> <td></td> </tr> </table> <p>0.1% AEP (1 in 1000 year) plus 65% climate change event:</p> <table style="width: 100%; border: none;"> <tr> <td>Proportion - 56%</td> <td>Mean Depth - 0.28m</td> </tr> <tr> <td>Max Depth - 0.82m</td> <td>Mean Velocity - 0.87m/s</td> </tr> <tr> <td>Max Velocity - 2.07m/s</td> <td>Mean Hazard - 1.08</td> </tr> <tr> <td>Max Hazard - 2.1</td> <td></td> </tr> </table>	Proportion - 5%	Mean Depth - 0.11m	Max Depth - 0.43m	Mean Velocity - 0.55m/s	Max Velocity - 0.96m/s	Mean Hazard - 0.61	Max Hazard - 1.24		Proportion - 16%	Mean Depth - 0.12m	Max Depth - 0.49m	Mean Velocity - 0.65m/s	Max Velocity - 1.54m/s	Mean Hazard - 0.64	Max Hazard - 1.33		Proportion - 56%	Mean Depth - 0.28m	Max Depth - 0.82m	Mean Velocity - 0.87m/s	Max Velocity - 2.07m/s	Mean Hazard - 1.08	Max Hazard - 2.1	
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Flood risk management infrastructure		
Existing defences	The Environment Agency’s AIMS dataset shows there are no formal flood defences within the vicinity of the site.	
Emergency planning		
Flood warning	The majority of the site has been identified to be located within an area of flood alerts for the River Axe area for the Rivers Axe, Coly, Yarty, Umborne Brook and coastal streams from Branscombe to Axmouth. This area of the site is also within the River Coly from Colyton to Colyford flood warning area. Mapping: Coly_06 - Flood Warnings and Alerts	
Access and egress	Access and egress is shown to be largely unaffected during all assessed events, with depths of up to 0.32m in a small, localised area on the B3161 to the west of the site shown in the 1% AEP plus climate change surface water modelling.	
Requirements for drainage control and impact mitigation		
Broad-scale assessment of possible SuDS	<p>Geology and Soils</p> <p>The geology consists of mudstone, siltstone and sandstone, with clay, silt and sand superficial deposits. The soils are shown to be loamy and clayey floodplain soils with naturally high groundwater across the majority of the site, with slightly acid loamy and clayey soils with impeded drainage to the west. This suggests that infiltration is unlikely to be a viable means of surface water disposal. The infiltration potential of the site should be confirmed through infiltration testing, in line with BRE 365.</p> <p>SuDS</p> <ul style="list-style-type: none"> • The site has not been identified to be located within a historic landfill site, a groundwater Source Protection Zone or Nitrate Vulnerable Zone. • Groundwater levels on site are close to the grounds surface, and therefore infiltration on site is unlikely. The infiltration potential of the site should be confirmed through infiltration testing, in line with BRE 365. Offsite discharge may therefore be required to discharge surface water runoff. • Surface water discharge rates should not exceed pre-development discharge rates for the site and should be designed to be as close to greenfield runoff rates as reasonably practical in consultation with the LLFA. It may be possible to reduce site runoff by maximising the permeable surfaces on site using a combination of permeable surfacing and soft landscaping techniques. • SuDS measures should follow the discharge hierarchy, and if it is proposed to discharge runoff to a watercourse or sewer system, the condition and capacity of the receiving watercourse or asset should be confirmed through surveys and the discharge rate agreed with the asset owner. • Due to the topography, any surface water not intercepted via infiltration will drain via gravity to the southeastern corner. It is therefore 	

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	recommended that the LLFA and the EA are consulted about viable discharge locations for surface water from the site and their attenuation potential.	
Opportunities for wider sustainability benefits and integrated flood risk management	<ul style="list-style-type: none"> • Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could also provide wider sustainability benefits to the site and surrounding area. Proposals to use SuDS techniques should be discussed with relevant stakeholders (LPA, LLFA and EA) at an early stage to understand possible constraints. • The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development. • Opportunities to incorporate source control techniques such as green roofs, permeable surfaces and rainwater harvesting must be considered in the design of the site. • SuDS are to be designed so that they are easy to maintain, and it should be set out who will maintain the system, how the maintenance will be funded and should be supported by an appropriately detailed maintenance and operation manual. • SuDS should be designed with a holistic approach, combining ecology, landscape and drainage requirements specific to the site, incorporating Biodiversity Net Gain requirements. • Opportunities to incorporate filtration techniques such as filter strips, filter drains and bioretention areas must be considered. Consideration should be made to the existing condition of receiving waterbodies and their Water Framework Directive objectives for water quality. The use of multistage SuDS treatment will improve water quality of surface water runoff discharged from the site and reduce the impact on receiving water bodies. • The potential to utilise conveyance features such as swales to intercept and convey surface water runoff should be considered. Conveyance features should be located on common land or public open space to facilitate ease of access. • SuDS should be designed in line with Devon County Councils SuDS Guidance. https://www.devon.gov.uk/floodriskmanagement/document/sustainable-drainage-system-guidance-for-devon-2023/#dcc-documents-cpt-contents 	
NPPF and planning implications		
Exception Test requirements (Local Authority considerations)	<p>The Local Authority will need to confirm that the Sequential Test has been carried out in line with national guidelines. The Sequential Test will need to be passed before the Exception Test is applied.</p> <p>The NPPF classifies the usage as “More Vulnerable”; this type is taken into consideration for the Exception Test.</p> <p>The site is predominantly located within Flood Zone 2 and 3, with a limited surface water extent in the 0.1% AEP. It is however suggested that development is proposed within Flood Zone 1 and 2 on site, due to the steep topography highlighting the potential inaccuracies of the historical data within</p>	



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Zone 3 detailed flood modelling should be undertaken within a site-specific FRA. Any development within Flood Zone 3 should be allocated as an undeveloped open space corridor, and not as gardens, car parking or other features associated with individual plots. It should also be noted that the site should not discharge surface water into combined sewer.

- Infiltration rates are assessed on site as part of a drainage strategy.
- Further investigation of existing drainage and highway connections should be undertaken to identify the risk to the site and associated flow paths.
- The Exception Test is satisfied and detailed modelling is undertaken on site as part of a site-specific FRA.
- Cumulative Impact Assessment policy documents must be understood, and the cumulative impact of development should be considered.